



GLOBAL JOURNAL OF RESEARCHES IN ENGINEERING: E
CIVIL AND STRUCTURAL ENGINEERING

Volume 19 Issue 2 Version 1.0 Year 2019

Type: Double Blind Peer Reviewed International Research Journal

Publisher: Global Journals

Online ISSN: 2249-4596 & Print ISSN: 0975-5861

Effect of Impact Load on SIFCON

By Nadia Moneem Al-Abdalay, Husein Ali Zeini & Huda Zuhair Kubba

Furat Al-Awsat Technical University

Abstract- In present paper, some mechanical properties of SIFCON specimens are studied and they are compared with conventional mortar. The cement based slurry used in this work is a composition of cement, fly ash, silica fume.

From the results achieved it was found that SIFCON specimens are much better than conventional mortar. The compressive, splitting, flexural strength and impact resistance for conventional mortar and SIFCON specimens were investigated and also plotted. Thus this study shows that SIFCON can be used as an effective alternative in special construction purposes or where the concrete or conventional steel fiber reinforce concrete cannot perform as expected/required or in situations where such high strength is important.

Keywords: SIFCON, micro steel fiber, fly ash, silica fume, impact resistance.

GJRE-E Classification: FOR Code: 290899P



Strictly as per the compliance and regulations of:



Effect of Impact Load on SIFCON

Nadia Moneem Al –Abdalay ^α, Husein Ali Zeini ^σ & Huda Zuhair Kubba ^ρ

Abstract- In present paper, some mechanical properties of SIFCON specimens are studied and they are compared with conventional mortar. The cement based slurry used in this work is a composition of cement, fly ash, silica fume.

From the results achieved it was found that SIFCON specimens are much better than conventional mortar. The compressive, splitting, flexural strength and impact resistance for conventional mortar and SIFCON specimens were investigated and also plotted. Thus this study shows that SIFCON can be used as an effective alternative in special construction purposes or where the concrete or conventional steel fiber reinforce concrete cannot perform as expected/required or in situations where such high strength is important.

Keywords: SIFCON, micro steel fiber, fly ash, silica fume, impact resistance.

1. INTRODUCTION

There are several new type of concrete that are being presented to enhance the strength of the concrete. In such case SIFCON, as a special type of steel fiber reinforced concrete (FRC), has a discrete steel fiber matrix which lends important tensile properties to the composite matrix and because of its high steel fiber content, SIFCON has also unique and superior properties in the areas of both ductility and energy absorption. The major dissimilarity between FRC and SIFCON, in addition to the differences in steel fiber volume fraction and method of production, is the absence of coarse aggregates in SIFCON mortar which, if used, will prevent the infiltration of the mortar through the steel fiber network. Also, SIFCON contains high amount of cement in comparison with conventional concrete and (FRC) [1,2]. The process of preparing SIFCON is different, because of high steel fiber content. SIFCON is produced by first poured steel fibers into a mold until it is completely filled. The steel fiber network of SIFCON is then infiltrated by cement-based slurry. While in (FRC) the steel fibers are mix intimately with wet mix of concrete, prior to mix being sprinkling into forms. The volume of steel fibers (Vf.) is a function of many parameters, such as the shape, diameter, and aspect ratio of steel fibers; their orientation; mold size; placement technique; and the extent of vibration.

External vibration can be applied during the steel fiber placement process. The stronger the vibration, the higher achievable Vf. Some researchers casted SIFCON members layer by layer under vibration to ensure whole infiltration of slurry into the steel fiber pack[3]. Three techniques for incorporating the steel fibers in the matrix was used by[4] to produced SIFCON. In the first case, the steel fibers were repacked in the molds and the slurry was allowed to infiltrate the steel fiber pack, assisted by a table vibrator (single-layer technique). The second technique involved initial placing and packing of the steel fibers in the mold only up to one-third depth, followed by infiltration of the slurry up to this level. The contents in the mold were then vibrated. The process was repeated until the entire mold was filled and compacted (three-layer technique). The third technique consisted of filling the mold up to one-third depth by the slurry, implanting the steel fibers into it immediately thereafter, vibrating the contents and repeating the process until the mold was full (immersion technique). The researchers found that the three techniques used for incorporating steel fibers in the mortar slurry proved effective during the casting of the SIFCON specimens. However, the three-layers and immersion techniques were found to be easier and simpler in real practice than the single-layer technique. Also he studied SIFCON specimen's behavior under impact loads. The impact test was carried out using the test rig. The weight of drop was 50 kg, and The drop height was varied from (250 – 1000) mm. The test results show that, the extent of damage in SIFCON due to impact load was found to be far less when compared to plain mortar and normal fiber reinforce mortar. The SIFCON process needs special attention relating mostly to the need of avoiding non-uniform fiber distributions and of avoiding unfavorable fiber orientation. The fiber density at the interior can be much higher, compared to the edges. Also, a number of fibers may align vertically along the outer surface. One method to escape the edge effect and fiber orientation problems is to cast a slab and get the test specimens by coring. Also, care should be paid to the orientation of fibers. If fibers are aligned along the diameter of the cylinder, a much higher compressive strength can be expected related to a cylinder in which fibers are aligned along the axis of the cylinder [5]. The behaviour of SIFCON slabs under impact loading was studied by[6]. The test was conducted by using impact testing machine with steel ball drop weight. The results reveals that SIFCON slabs with 12% fiber content shows excellent performance in strength and toughness characteristics compared to fiber reinforced concrete,

Author ^α : Civil Department of Najaf Technical Institute at the Furat Al-Awsat Technical University, Iraq. e-mails: nadia.material@gmail.com, inj.nad@atu.edu.iq

Author ^σ : Civil Department of Najaf Technical Institute at the Furat Al-Awsat Technical University, Iraq. e-mail: inj.hus@atu.edu.iq

Author ^ρ : Civil Department of Babylon Technical Institute at the Furat Al-Awsat Technical University, Iraq. e-mail: Huda Zuhair Kubba@gmail.com

reinforced cement concrete and plain cement concrete slab specimens. [7] studied the experimental investigations on partial substitution of cement with fly ash in concrete mix design. Cement was partially replaced with fly ash in the range of 0%, 10%, 20%, 30% and 40% for making concrete mix design normal and high strength concrete mix. The compressive strength and splitting tensile strengths were reduced with increased percentage of fly ash content, but cost of concrete decreased due to reduction of quantity of cement. [8] studied the mechanical properties and flexural properties of SIFCON specimens and compared with conventional concrete of grade M40. The cement based slurry is a composition of cement, fly ash, silica fume, Ground Granulated Blast Furnace Slag. From the results obtained it is found that SIFCON specimens are much better than conventional concrete. [9] investigated the effect of several factors on the impact resistance of SIFCON. These factors are; fiber volume fraction (6%, 8.5%, and 11%), SIFCON mortar type (using silica fume and/or fly ash as a replacement of cement), and different fiber type was using (hooked end fiber, micro steel fiber and hybrid fiber which are varies in their aspect ratio (l/d) and geometry). A conventional fiber reinforced mortar with 2% hooked fiber content is also produced as a control (reference) mix to be compared with SIFCON mixes. The impact resistance of SIFCON specimens was carried out using disc specimen (152 mm diameter by 63 mm thick). the results show that in

general, SIFCON mixes exhibited higher mechanical properties as compared with reference mix.

II. EXPERIMENTAL PROGRAM

The experimental work was carried out by casting cubes of size $100 \times 100 \times 100$ mm to find the compressive strength, prism of size $100 \times 100 \times 400$ mm to investigate the flexural strength, cylinder of 100 mm diameter and 200 mm height were casted to obtain the splitting tensile strength and plate of size $500 \times 500 \times 40$ mm to find impact resistance. The edges of the mold were sealed with plaster of Paris to prevent the leakage of slurry. The micro steel fiber is dispersed in a random manner to the volume fraction. Compaction by table vibrator was used to ensure complete penetration of the slurry into the micro steel fiber pack. After twenty-four hours of casting, the cubes were demolded and cured in water for the test day.

III. MATERIALS USED

a) Cement

Ordinary Portland cement which is commercially known as (Krasta), produced in Sulaymaniyah, was utilized in this study. Its chemical composition and physical properties are presented in Table (1), the results show that the cement utilized conforms to the Iraq specification No. 5/ 1984 [10].

Table 1: Chemical composition and main compounds of the cement used

Oxide	%	I.Q.S. 5: 1984 ^[10] Limits
CaO	66.11	—
SiO ₂	21.93	—
Al ₂ O ₃	4.98	—
Fe ₂ O ₃	3.10	—
MgO	2.0	< 5.0
K ₂ O	0.75	—
Na ₂ O	0.35	—
SO ₃	2.25	< 2.8
Loss on Ignition (L.O.I)	2.39	< 4.0
Lime Saturation Factor	0.93	0.66 - 1.02
(L.S.F) Insoluble residue (I.R)	1.29	< 1.5 %
Free lime (F.L)	0.67	—
Compound Composition	%	I.Q.S. 5: 1984 ^[10] Limits
C ₃ S	58.16	—
C ₂ S	19	—
C ₃ A	7.95	—
C ₄ AF	9.43	—
Physical Properties	Test Results	I.Q.S.5:1984 ^[10] Limits
Fineness, Blaine, cm ² /gm	3300	> 2300
Setting Time:		
Initial hrs.; min	1;08	≥ 45 min
Final hrs.; min	4;00	≤ 10hrs
Compressive Strength MPa		
3-days	20,0	≥ 15
7-days	25,0	≥ 23

b) *Fine aggregate (sand)*

Locally available river sand passing through 4.75 mm sieve was used. The specific gravity was found as 2.62. Table (2) shows the sieve analysis and the

grading curve of the used sand. It conforms to the limits of Iraq specification No. 45/1984[11], Zone (2). The chemical and physical properties of natural sand are illustrated in Table (3).

Table 2: The sieve analysis of the used sand

Sieve size (mm)	Passing by weight%	I.Q.S.45/1984 [11] Zone(2)
9.5	100	100
4.75	98.5	90-100
2.36	92.4	75-100
1.18	82.0	55-90
0.6	47.5	35-59
0.3	14.4	8-30
0.15	3.2	0-10
pan	2.23	<5

Table 3: Properties of the used sand

Properties	Test results	I.Q.S.No.45 [11]/1984
Specific gravity	2.6	-
Bulk density(kg/m ³)	1670	-
Sulfate content%	0.34	≤ 0.5%
Absorption	2	-

c) *Supplementary Cementitious Material*

i. *Silica Fume*

Silica fume conforming to ASTM C 1240[12] was utilized in this study as a partial replacement (10%)

by weight of cement. The technical specifications of silica fume are presented in Table (4).

Table 4: The technical specifications of silica fume

Structure of material	Silica fume	Limits of ASTM C 1240-05 [12]
Color	Dark gray	
Density	0.55-0.7 kg/m ³	
Chlorine amount	< 0.1 %	
Specific surface area (cm ² /gm)	> 150000 cm ² /g	≥ 150000 cm ² /g
SiO ₂	> 85 %	≥ 85 %
CaO	< 1 %	
Activity index	156 %	≥ 105 %
Specific gravity	2.2	

ii. *Fly Ash*

Class F fly ash (FA) produced from Thermal Power plant in Turkey is used as an additive according

to ASTM C 618 [13], cement is replaced by (20%) of fly ash by weight of cementitious material. The physical and chemical properties are presented in Table (5).

Table 5: Physical and chemical properties of class (F) fly ash

Particular	Fly ash (Class F)	ASTM C 618 Class F fly ash[13]
Chemical composition		
Silica (SiO ₂)%	65.65	(SiO ₂ + Al ₂ O ₃ + Fe ₂ O ₃) ≥ 70
Alumina (Al ₂ O ₃)%	17.69	
Iron Oxide (Fe ₂ O ₃)%	5.98	
Lime (CaO)%	0.98	
Magnesia (MgO)%	0.72	
Sulphur Trioxide (SO ₃)%	0.19	Max. 5.0
Loss on Ignition	3.1	Max. 6.0
Na ₂ O	1.35	
K ₂ O	2.98	
Physical properties		
Specific gravity	2.12	
Fineness (cm ² /gm)	3600	Min. 2250cm ² /gm

d) *High range water reducing admixture (HRWR)*

(Glenium 54), from BASF company, was used as a HRWR. It is free from chloride and compiles with ASTM C494-05 Type F [14]. Glenium 54 is based on a carboxylic ether polymer with long later chain which

greatly improves cement dispersion at the start of mixing process. HRWR had a dosage of 2liters per 100 kg of cementitious materials, Table (6) shows the main properties of the (HRWR).

Table 6: Typical properties of SP (Glenium54) *

Form	Viscous Liquid
Commercial name	Glenium 54
Chemical composition	Sulphonated melamine and naphthaline formaldehyde condensates
Appearance	Whitish to straw colored liquid
Relative density	1.07 gm/cm ³ at 20 °C
Chloride content	Nil.
pH	5-8
Storage	Should be stored in original containers and at above 5 °C
Transport	Not classified as dangerous
Labeling	Not hazard label required
Alkali content (as NaO ₂ equivalent)	0.26%

*According to manufacturer

e) *Water*

Fresh water available from local sources was used for mixing and curing of specimens.

f) *Micro Steel Fiber*

Micro steel fibers are used in SIFCONs to enhance some properties and improve the ductility, the properties are summarized in Table (7). Micro steel fiber (6%) volume fraction V_f was randomly distribution by hand.

Table 7: Specification of Micro Steel Fiber*

Property	Specification
Type	WSF 0213
Surface	Brass coated
Tensile Strength	2850MPa
Length	15mm
Diameter	0.2mm
Aspect ratio	65

*According to manufactu 4- Mix:

The proportion of the constituents for the prepared slurry mix is 1:1 (by weight) of ordinary Portland cement and cementitious materials (Silica fume and Fly ash) to sand, while the water/binder ratio was kept constant as 0.3 (by weight). The super plasticizer (SP) had a dosage of 2liters per 100 kg of cement materials, (6%) volume fraction of micro steel fiber was choosing to reinforced SIFCON mix. A conventional micro steel fiber reinforced mortar, with (2%) fiber volume fraction, was also prepared as a comparison (reference) mix with SIFCON mix. The mixing procedure used to produce SIFCON was as described:

- Before mixing operation, the mixer was cleaned off from any remaining fresh or hardened materials from the older mix.
- Firstly, add cement, fly ash and silica fume while mixer operating at low speed for 30 sec. until a uniform distribution is reached,
- Secondly, add sand and mixing for 1 min. at medium speed,0
- Thirdly, first part (2/3) of water was added and mixed thoroughly for 30 sec. at low speed,
- stop 2 min to clean blades,
- Now, adding SP and remainder water and mixing for 2 min. at normal velocity,
- Stop the mixer and wait for 1 min., and then finalize the process by mixing at normal velocity for 3 min. After many trails of casting technique in the laboratory, two-layer technique was used for incorporating the steel fiber into the SIFCON.

According to reference [9] two-layer technique was utilized for combining the micro steel fiber into the SIFCON matrix. The two - layer technique involved primary placing and packing the micro steel fiber which were oriented in random method, in the mold only up to half depth, followed by filling the mold by the slurry up to half depth. The slurry has to be flow able enough to ensure infiltration through the micro steel fiber. This procedure was repeated (for the second layer) where the entire mold was filled with the required volume fraction of fiber. No vibration was applied.

IV. TESTS FOR FRESH SIFCON MORTARS

a) *Determination of Slump-Flow*

The test apparatus for measuring the flow and viscosity of mortar the dimension of slump – flow is show in Fig. (1).The subsequent diameter of the mortar

is measured in two perpendicular directions and the average of the diameters is stated as the spread of the mortar. In this test, the truncated cone mold is sited exactly on the (100mm) diameter graduated circle marked on the glass plate, filled with mortar and lifted

upwards. Fresh properties of mortars were calculated by the mean value two perpendicular flow diameters in the spread test. The procedure for test was followed as described in [15].

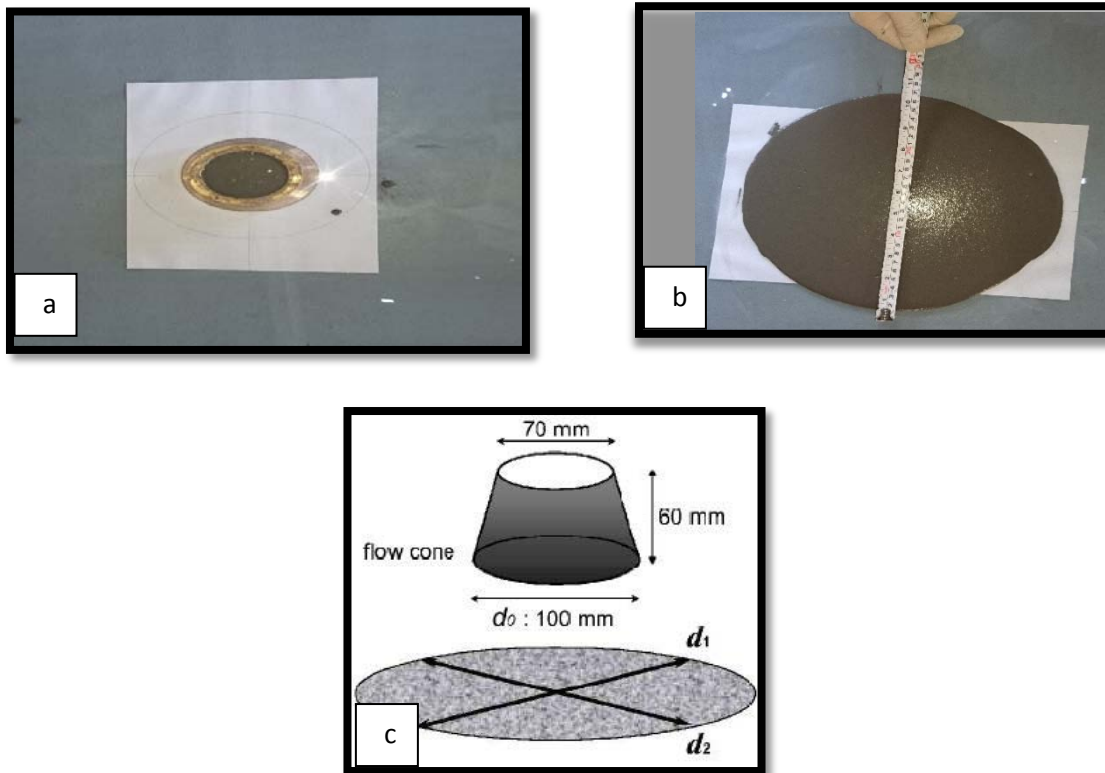


Fig. 1: Mini slump flow test (a, b), (c) the apparatus used [15]

b) Determination Flow Time

Flow time determined in the v-funnel test, the dimension of v- funnel is show in Fig. (2).

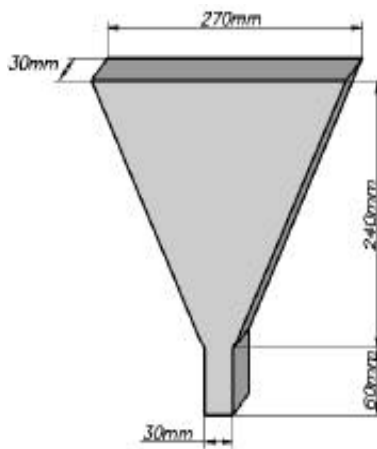


Fig. 2: V-Funnel test apparatus [15]

V. TESTING OF HARDENED SIFCON

a) Compressive strength test

The compressive strength test was carried out on 100 mm cube, by using a 2000 kN capacity hydraulic testing machine type ELE digital testing. The loading rate was applied at 0.3 N/mm² per second according to BS.1881: part 116[16]. The Specimens were tested at the ages of 7, 28, and 90 days of water curing. The average of three specimens was recorded for each variable in this test.

b) Splitting tensile strength test

Splitting tensile strength test was carried out according to ASTM C496-04[17]. Cylindrical specimens of 100×200 mm were employed and the average result of two specimens at ages of 7,28, and 90 days was taken for each mix. The splitting tensile strength can be calculated from equation (1). Splitting tensile strength test was made by ELE Digital Elect testing machine. The loading rate used in the test is 0.3 N/mm² per second.

$$F_{st} = 2P/\pi LD \quad (1)$$

Where:

F_{st} : Splitting tensile strength (MPa). P: applied load (N).
D: diameter of the specimens (100mm). L: length of the specimens (200mm).

c) *Flexural strength (modulus of rupture)*

This test was performed in accordance with ASTM C1609-12[18], using prismatic specimens of 100×100×400mm simply supported beam. The specimens were tested under two point loads with a constant rate of loading about 0.015MPa/sec. The specimens were tested at ages of 7, 28 and 90 days and the average of two specimens was recorded. The flexural strength (modulus of rupture) was calculated using the following formula:

$$f_r = \frac{PL}{bd^2} \quad (2)$$

where

P: maximum applied load, (N). L: Span length of specimen, (mm).

b: the width of the specimens, (mm). d: the depth of the specimen, (mm)

d) *Impact resistance test*

Steel molds with (500×500×40mm) are fabricated for casting the plates specimens. The molds are made of (4 mm) thickness steel and their side pieces are connected by bolts which can simply be removed and fastened. The same procedure casting for other SIFCON test was utilized for impact resistance. Fig (3) show the impact resistance mold during casting.

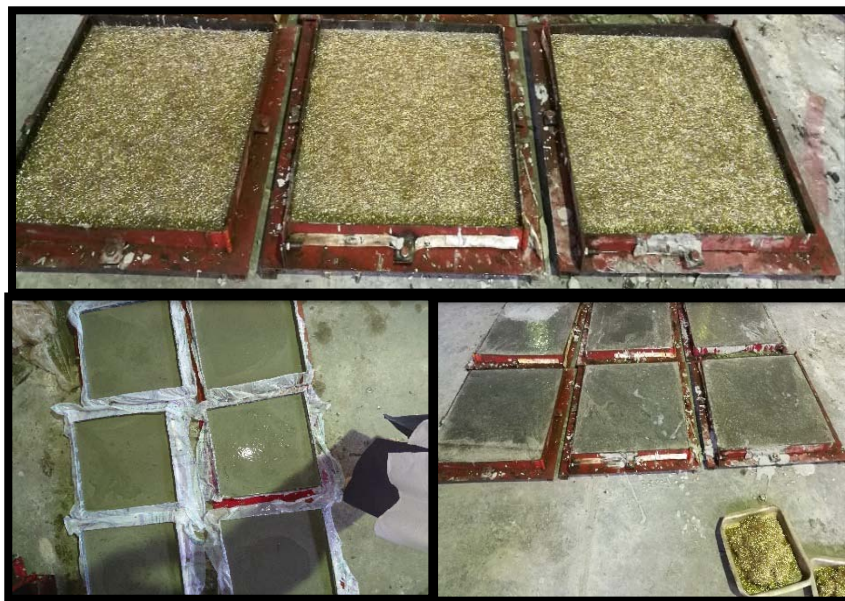


Fig. 3: Impact resistance mold during casting. a) micro steel fiber pack, (b) during slurry cast, (c) after hardened specimens

A steel frame was manufactured, for this test and the instrument Schematic diagram and picture was presented in Fig. (4). The test procedure adopted is as (5kg) steel mass was released from a height of (1m) repeatedly, which would come in contact with the top surface of the center of plate specimens. The number of impact blows until the appearance of first visible crack was recorded. The loading was then continued and the number of blows until failure was recorded. In average, three plate specimens are adopted in this instrument for 90 days' age. The energy absorption value was obtained by:

$$E = N \times (w \times h) \quad (3)$$

Where; E is the energy in joules, w is weight in Newton, h is the drop height in meter and N is the blows in numbers.

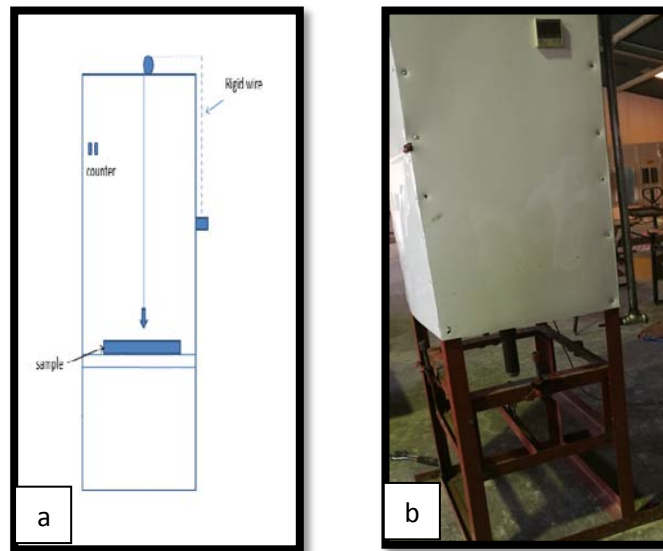


Fig. 4: Impact test apparatus, (a) Schematic diagram, (b) impact test instrument.

VI. RESULTS AND DISCUSSION

a) Fresh SIFCON Properties

The test results related to the slump flow diameter, V-funnel flow time are presented in Table (8). From result show the mixture had slump flow diameter, V-funnel flow time conforming [15]. Where D_m is the

mean value of the two perpendicular diameters, measured in (mm); D_0 is the initial diameter of the base of the cone, measured in (mm), and finally, the (t) represents the time of flow in the v-funnel, which is measured in second.

Table (8): Fresh properties of SIFCON mortars*

Slump Flow Diameter (cm)	V-funnel time (s)	$G_m = \left(\frac{D_m}{D_0}\right)^2 - 1$	$R_m = \frac{10}{t}$
25.8	8	5.65	1.25

* The EFNARC [15] requirements are: a value between (240-260) mm spread diameters is required for mini slump flow test and a flow time between (7-11) seconds for V-funnel test

b) Hardened SIFCON Properties

i. Compressive Strength

The compressive strength test results for SIFCON cubes $100 \times 100 \times 100$ mm cured in water until days of test at age of 7, 28 and 90 days, from the results showed in Table (9) and Fig. (4), the strength increased with ages. This development in compressive strength can be attributed to the fact continuous the hydration process (C-S-H), also present of silica fume tends mainly to consume the calcium hydroxide crystals released from the hydration process leading to the creation of further calcium-silicate-hydrate (secondary C-S-H).

The increase in fiber fraction from 2% (reference) to 6% (SIFCON) mix leads to enhance the compressive strength of SIFCON mix to (73.2, 75.4 and 65.3%) at age of 28, 60 and 90 days respectively. This enhance in compressive strength is due to the developed gained bond between micro steel fiber/matrix interfaces by increasing micro steel fiber V_f , in addition to the effect of micro steel fiber in bridging the microcracks growth, and therefore lead to higher strength of the composite, these results are in agreement with other researchers [3, 4, 5, 9].

Table (9): Results of Hardened Properties of SIFCON Mix

Group NO.	Mix Symbol	Compressive Strength (MPa)			Splitting Strength (MPa)			Flexural Strength (MPa)		
		7 days	28 days	90 days	7 days	28 days	90 days	7 days	28 days	90 days
Reference	R	55	62.3	72.1	5.5	7.1	9.9	7.6	9.9	11.0
SIFCON	S	95.3	109.3	119.2	16.1	19.8	21.9	18.0	24.3	28.0

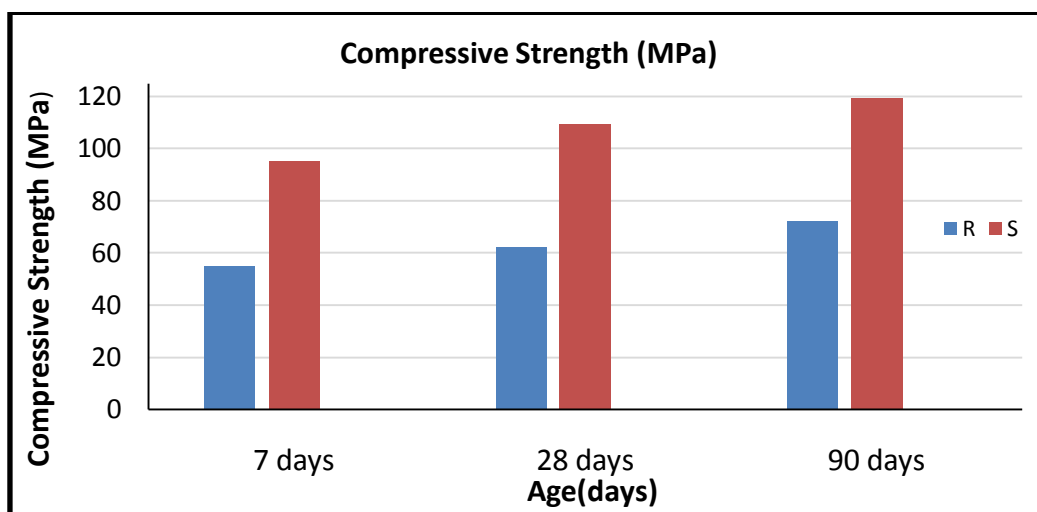


Figure (4): Compressive Strength of SIFCON Mix.

ii. Splitting Strength

The tensile strength values of SIFCON mix were carried out at the different ages of 7, 28, and 90 days, are presented in Table (9) and plotted in Fig. (5). It shows that, SIFCON specimens have superior tensile properties, important enhance in tensile strength by about (1.92, 1.81 and 1.21%) at age of 7, 28 and 90 days respectively. This enhanced can be attributed to the

microcracks can be controlled by arresting and bridging mechanism of fiber. Also the using micro steel fiber leads to an improved bond between fiber and matrix, hence improvement in mechanical properties of SIFCON. This result is in agreement with other researchers [3, 4, 5, 9].

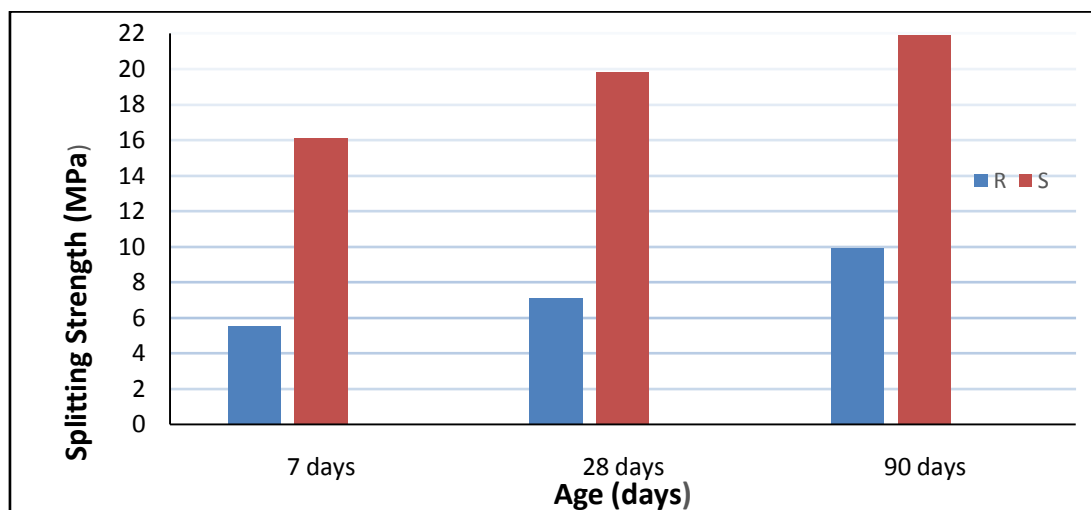


Figure (5): Splitting Tensile Strength with Age for SIFCON Mix

iii. Flexural strength

The flexural strength values of SIFCON specimens at different ages for SIFCON mix and also the flexural strength of reference mix, are presented in Table (9) and presented in Fig. (6). It is obvious that, important enhance in flexural strength by about (1.36, 1.45 and 1.54%) at age of 7, 28 and 90 days respectively, when comparing the results of SIFCON specimens with that of reference specimens. This result can be attributed to the stronger and greater interface zone between binder and micro steel fibers which enhances

the bond strength and decreases the growth of microcracks which leads to flexural failure. This result is in agreement with other researchers [3,4, 5,9].

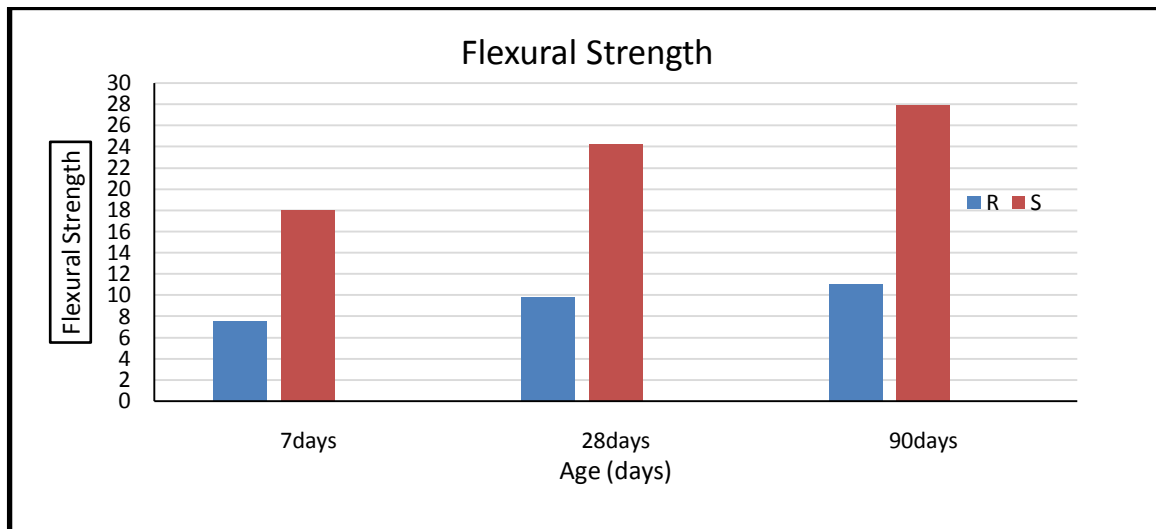


Figure (6): Flexural Strength with Age for SIFCON Mix

iv. Impact Resistance

The results of number blows required to first and failure crack at age of 90 days are present in Table (10) and Fig. (7). The test results show that a significant development in impact resistance for SIFCON mix at first crack and failure compared with the conventional mortar (reference) mix by about 8.77%, 8.97% for initial crack and complete failure respectively. SIFCON mix exhibited the highest impact resistance, and the energy required for complete failure was (65089.35 joules)

which is increased by 9.97 times, compared to the reference mix at 90 days as show in Table (10) and Fig. (8). Also this result was much higher than that of high performance fiber reinforced concrete which is ranged between (3000- 50000 joules)[19, 20]. This result is in agreement with other researchers [4, 9], and can be attributed to the ability of micro steel fibers in absorbing high amount of impact energy because of its high tensile strength and high ductility.

Table (10): Impact resistance for SIFCON specimens at 90 days' age.

Group No.	Mix symbol	Number of blows to cause		Impact energy (joules)		Residual impact strength ratio (Irs)
		Initial Crack	Complete Failure	Initial	Failure	
Reference	R	44	133	2158.2	6523.65	3.02
SIFCON	S	430	1327	21091.5	65089.35	3.08

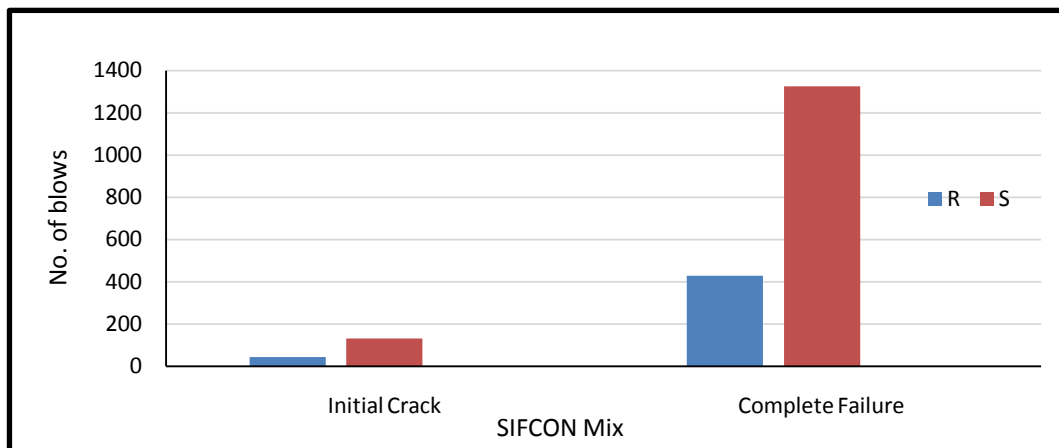


Fig. (7): Number of blows required to cause first crack and complete failure of SIFCON mix

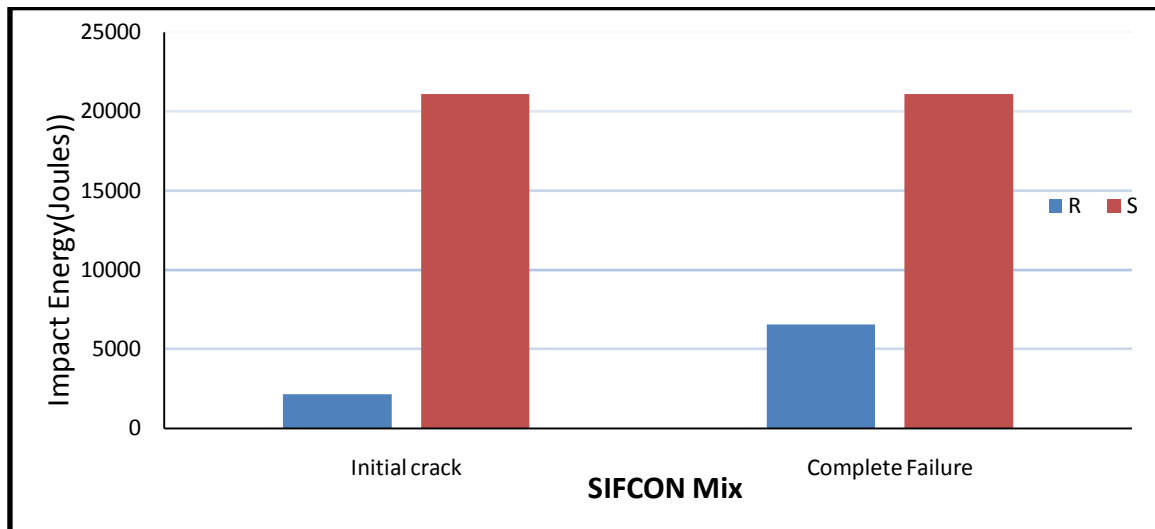


Fig. (8): Impact energy (joules)

VII. CONCLUSION

From the experimental study of SIFCON, the following conclusion is drawn.

1. The (compressive, splitting, flexural) strength increased with ages for conventional mortar (reference) and SIFCON mix.
2. SIFCON show enhance in the (compressive, splitting, flexural) strength for all ages test when comparing with conventional mortar (reference).
3. a significant development in impact resistance for SIFCON mix at first crack and failure compared with the reference mix.
4. SIFCON mix exhibited the highest impact resistance.
5. The energy required for complete failure was which is increased by 9.97 times compared to the reference mix.

REFERENCES RÉFÉRENCES REFERENCIAS

1. Gilani, A.M., "Various durability aspects of slurry infiltrated fiber concrete", Ph.D. thesis, the graduate school of natural and applied sciences of middle east technical university, Turkey, pp. 209, 2007.
2. Yazıcı, H., Aydın S., Yiğiter, H. Yardımcı, M. Y., and Alptuna, G., "Improvement on SIFCON performance by fiber orientation and high-volume mineral admixtures", Journal of materials in civil engineering, Vol. 22, Issue 11, November, pp.1093-1102, 2010.
3. Krishnan, M. G. , "Experimental study on slurry infiltrated fibrous concrete with sand replaced by Msand ", Department of Civil Engineering, SASTRA UNIVERSITY, Vol. 3, Issue 5, India, pp.534-538, May 2014.
4. Parameswaran, V. S., Krishnamoorthy, T. S., Balasubramanian, K and, Gangadar, S. , "Studies on slurry-infiltrated fibrous concrete (SIFCON)", Structural Engineering Research Center, National Academy of Sciences, Issue No. 1382, part 2, India, pp.57-63., 1993.
5. Giridhar R., Rama P. and Rao M., "Determination of mechanical properties of slurry infiltrated concrete (SIFCON)", International Journal for Technological Research in Engineering Vol. 2, Issue 7, pp. 1366-1368, March-2015.
6. Sudarsana, H Rao, Vaishali G.Ghorpade, NV Ramana and K.Gnaneswar 2008. "Response of SIFCON two-way slabs under impact loading International Journal of Impact Engineering" Vol.37, pp.452-458.
7. Jayeshkumar Pitroda, L. B. Zala, F.S.Umrigar.2012. "Experimental investigations on partial replacement of cement with fly ash in design mix concrete". International Journal of Advanced Engineering Technology, Vol. III, pp.126-129.
8. Pradeep, T., and Sharmila, S. , "Cyclic behavior of RC beams using SIFCON sections" International Journal of Innovative Research in Science, Engineering and Technology, Vol. 4, Issue 9, pp.9172-9177, September 2015.
9. Ali, M.A., " Properties of Slurry Infiltrated Fiber Concrete (SIFCON)", PHD Thesis, Department of Building and Construction Engineering of the University of Technology, 2018.
10. Iraq Standard Specification (IQS) No.5, "Portland cement", Baghdad, 8p, 1984.
11. Iraq Standard Specification (IQS) No.45, "natural sources of aggregate used in building and concrete", Baghdad, 13p. , 1984.
12. ASTM C 1240 – 05, "Standard specification for silica fume used in cementitious mixtures", American Society for Testing and Material
13. International, Vol.04.02, pp. 1-7, 2005.
14. ASTM C618-05, "Standard specification for fly ash used in cementitious mixtures", American Society for Testing and Material International, pp.1-3, 2005.

15. ASTM C 494-05, *"Standard specification for chemical Admixtures for concrete"*, American Society for Testing and Material International, 2005.
16. EFNARC *"Specification and guidelines for self-compacting concrete"*, European Federation for Specialist Construction Chemicals and Concrete Systems, Norfolk, U.K. ,pp. 1-32,2005.
17. British Standard 1881, Part 116, *"Method for determination of compressive strength of concrete cubes"*, British Standards Institution, 1881, pp.3, 1989.
18. ASTM C496-04, *"Standard test methods for splitting tensile strength of cylindrical"*, ASTM Standards, Vol. 04.02., PP. 283–287,2004.
19. ASTM C1609M–12, *"Standard test method for flexural performance of fiber-reinforced concrete (using beam with third point loading)"*, American Society for Testing and Material International, 2012.
20. Murali, G., Santhi A. S., and Mohan G.G. ,*"Empirical relationship between the impact energy and compressive strength for fiber reinforced concrete"*, journal of scientific and industrial research, India vol.73, pp.469-473, July 2014.
21. Kanagavel, R. ,and Kalidass, A.,*" Mechanical properties of hybrid fiber reinforced quaternary concrete "*, Original scientific paper DOI: <https://doi.org/10.14256/JCE.1282>, 2017.