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1	Finite Element Model for Prediction of Highway Pavement
2	Deformation
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6 Abstract

⁷ The determination of stresses developed in a pavement constitutes a basic prerequisite and is

 $_{\circ}$ achieved mainly by implementation of various methods which is dependent on the number of

⁹ distinct pavement layers. The need to predict the deformation of highway pavement with a

¹⁰ precision that will aid optimal design cannot be oversized. Boussinesq?s work was

¹¹ foundational for the development of all subsequent elasticity theories, but Boussinesq assumed

¹² one layer of uniform subgrade material. In this research, a mechanistic elastic model for

¹³ obtaining deformation in road pavement was derived using Finite Element Method (FEM).

¹⁴ This model was found to be an improvement on the Boussinesq model owing to the closeness

¹⁵ of its result to that obtained from Plaxis software. In addition to this, it has the capability of

¹⁶ handling deformation in both flexible and rigid pavement utilizing the dimensional similarities

¹⁷ between unit weight and modulus of subgrade reaction of soil. A MATLAB program was also

- ¹⁸ written for easy computation using the new model.
- 19

20 Index terms— pavement deformation; finite element model; boussinesq?s model; MATLAB program.

²¹ 1 I. Introduction a) Causes of Pavement Deformation in High-

22 way

Pavement eformation of highway pavement can be occasioned by weak soils [1][2], frost action [3][4], expansive soils [5], Unbound aggregate material [6], seasonal drying and wetting [7]. Deformation can also result from thermal stresses [8], differential subgrade settlement [10], and aggregate morphology [11][12].

²⁶ 2 b) Methods of Analysis of Highway Pavement

Boussinesq's work was foundational for the development of all subsequent elasticity theories. Boussinesq's theory assumed one layer of uniform and homogenous subgrade material. According to [13], the stresses applied to an elastic homogenous and isotropic material extended to infinity at both directions, (horizontal and vertical) and the stress developed at any depth, z, below the surface of the pavement under the influence of a point load in Figure ?? can be calculated thus:

Vertical stress,? ?? = 3?? 2?? . ?? 3 ?? 5 (1)

After the pioneering work of Boussinesq, different methods of analysis have been used in obtaining stresses and 33 34 the accompanying deformation in highway pavement. Behera (2013) [14] used linear elastic theory in analyzing 35 the deformation behaviour of fly ash composite material in the subbase of surface coal mine haul road. Uzan 36 (2004) [15] applied the mechanistic framework in determining the permanent deformation of flexible pavement. Du and Dai (2006) [16] utilized the dynamic stability evaluation index in analyzing permanent deformation. It 37 was discovered that the method is not fit for evaluating permanent deformation of asphalt mixture. Tchemou et 38 al. 2011 [17] and Qiao et al. 2015 [18] applied rutting mechanisms in predicting flexible pavement degradation, 39 [19] used model simulation in determining permanent deformation in high-modulus asphalt having sloped and 40 horizontally curved alignment. Du and Shen (2005) [20] applied grey modelling method, [21] used field cores, and 41 [22] used ground-penetrating-ladar in predicting the development of irrecoverable deformation in road pavement. 42

Sawant (2009) [23] used dynamic analysis whereas [24] used the back-calculation of the transition probability
 approach. Each group of researchers demonstrated the merit of their method.

Many researchers have applied finite element method (FEM) in the analysis of deformation in highway 45 pavement [25][26][27][28]. He et al. (2008) [29] used 3D viscoelastic finite element analysis (FEA) in determining 46 asphalt pavement rutting deformation. Kim et. al. (2014) [30] used FEM in modelling the effect of environmental 47 factors on rigid pavement deformation. In analyzing the influence of asphalt deformation under heterogeneous 48 settlement of roadbed whereas [31] used elastic-plastic dynamic FEM to compute the differential settlement of 49 the half-filled and half dug embankment under axle load. The latter succeeded in deriving a model for computing 50 critical differential settlement. Each of the models is unique depending on the assumptions made by each group 51 of researchers. ??adek and Shahrour (2007) [32] compared Boussinesq's model with the occasional plastic nature 52 of subgrade and pavement materials. The researchers model was shown to be an improvement on Boussinesq's 53 model. 54

55 3 II. Purpose

This work involves the finite element method for predicting pavement deformation. Each model cited is derived either for rigid pavement and flexible pavement. However, this model is also unique owing to assumptions and approach was derived to handle both rigid and flexible pavement. Secondly, according to [33], many models used in the structural design of pavements are complex and/or difficult to use in the field, making its application in pavement analysis rather difficult. This model is devoid of such complexities.

⁶¹ 4 III. Methodology a) Derivation of the New Model i. Model ⁶² Assumption

63 In the derivation of the new model for deformation behaviour, the following assumptions were made;

1. Loading is symmetrical 2. Soil is elastic, homogenous and isotropic 3. The principle of superposition is valid 4. Constitutive law is valid 5. The idealized system of pavement structure is treated as a beam on elastic subgrade 6. The UDL from asphaltic concrete is converted to point load to produce the worst deformation needed for optimal design. 7. The problem is two-dimensional.

ii. Model Derivation A road of base course thickness ?? ?? , asphaltic concrete (AC) thickness as ?? ?? ,
and width l is subjected to a standard axle load ?? ?? as shown in Figure ??0. To determine the total stricture
stiffness matrix for a spring assemblage by using the force/displacement matrix relation of FEM, the model is
discretized into nodes and element as shown in Figure 3. ??3) is obtained.???? ?? 3 (1+? ??) ? 12 6?? ?12 6??
6?? (4 + ? ??) ?? 2 ?6?? (2 ? ??) ?? 2 ?12 ?6?? 12 ?6?? 6?? (2 ? ?)?? 2 ?6?? (4 + ? ??) ?? 2 ??? 1 ?? ?

73 1 ?? 2 ?? ? 2 = 0 ? = ? ? ?? 1 ?? 0 ???? +?? 2 ???? ?? 2 0 ? ? ? (6) K 2 1 2 3 (1) 0 K 1 ???? + ?? 2 ?? ?? 74 ?(2)

(1) ???? + ?? 2 ?? ?? ? 2 2 L 1 l List of Abbreviations ? z = Vertical Stress Q = Vertical Load Z = Vertical Load R =

77 5 Highlights

? The need to predict the deformation of highway pavement with a precision that will aid optimal design cannot
be overemphasized. Applying the boundary condition ?? 1 ?? = 0 = ? 2 ?????????????????????? using the 2 nd and
3 rd row of equation 13 whose rows are associated with the two unknowns, ? 1 and ?? 2 ?? and simplifying,
we obtain; (7) ?? 2 ?? = (???? + ?? 2 ?? ??)(4 + ? ??)?? 3 24???? For long slender beams with L about
10 times or more, the beam depth, shear correction term ? ?? is small and can be neglected [34]. For standard
highway, L=7.

⁸⁴ 6 IV. Conclusion and Recommendation

Many roads fail even before their design lives, probably because of using conservative models in their design to save cost. The cost implication of early maintenance and/or rehabilitation implies that using conservative models is not economical in the real sense. This new model, being close with the result from plaxis software shows that it is an improvement on Boussinesq's model which is found to be conservative. Secondly, the dimensional uniformity between unit weight and modulus of subgrade reaction was utilized by the researchers in making it a flexible model that can handle deformation in both rigid and flexible road pavement unlike many existing models.

⁹¹ 7 V. Declarations a) Ethical Approval and Consent to Partici-

92 pate

⁹³ The research observed all ethical codes and done with the consent of all authors involved.

⁹⁴ 8 b) Consent for Publication

95 We give our Consent for the publication of the article.

⁹⁶ 9 c) Availability of Supporting Data Not applicable d) Code ⁹⁷ Availability

98 Not applicable e) Funding Not applicable 1

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9 C) AVAILABILITY OF SUPPORTING DATA NOT APPLICABLE D) CODE AVAILABILITY

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